



PROPOSED CHILD CARE CENTRE

39 CAIRO AVENUE, REVESBY

TRAFFIC AND PARKING ASSESSMENT REPORT

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REF 20069

Prepared by

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1. INTRODUCTION

This report has been prepared to accompany a Development Application (DA) to Canterbury Bankstown Council for a proposed Child Care Centre at 39 Cairo Avenue, Revesby (Figures 1 and 2).

The development site is located on the north-eastern corner of the Sphinx Avenue/Cairo Avenue intersection. It has a total site area of 932.7m² with frontages of approximately 54m to Cairo Avenue and 19m to Sphinx Avenue.



Aerial photograph of the site

The existing site development comprises a single dwelling that gains vehicular access to Cairo Avenue via a single width access driveway located adjacent to the eastern site boundary.

The proposed development comprises the demolition of the existing dwelling and construction of a Child Care Centre containing 40 children and 6 employees. The Centre is expected to operate from 7.00am to 6.00pm Monday to Friday (excluding public holidays).



The Centre will be served by a total of 10 spaces comprising 5 staff spaces and 5 visitor spaces for child drop-off and pick up. Of that visitor parking provision, one space will be made available to disabled users.

A further 3 on-street visitor spaces will be provided on Cairo Avenue for convenient drop-off and pick up. As required by Council, these 3 spaces are to be located 10m north of the double white (BB) line on Cairo Avenue and will be signposted accordingly to Council's specifications. It is expected that Council will require a 15 minute restriction during the peak drop-off and pick up periods.



Location of proposed short-term parking spaces on Cairo Avenue

Vehicular access to the site is via a 6.1m wide combined entry/exit driveway off Sphinx Avenue located approximately 15m east of the Cairo Avenue intersection. This driveway will also be located 1.5m from the existing power pole on Sphinx Avenue as per the Ausgrid requirements and will be designed in accordance with Council's STANDARD MEDIUM DUTY VEHICULAR FOOTWAY CROSSING – Standard Drawing S-008. A copy of the Standard Drawing is reproduced in the following pages.

The existing driveway serving the dwelling will be made redundant.



The subject site has convenient access to the following bus service operating along Sphinx Avenue:

Route 962	East Hills to Miranda via Revesby, Padstow, Illawong, Menai, Bangor, Sutherland
	and Gymea. Service operates daily
Route S5	Padstow to Milperra via Revesby and Panania. Operates weekdays only

Bus stops are located on Sphinx Avenue in the vicinity of the site.

Plans of the proposed development prepared by MD+A Architects are reproduced in Appendix A.

The purpose of this report is to assess the traffic and parking implications of the proposed development.



















2. PARKING ASSESSMENT

Council Off-Street Parking Requirements

Section 2 in Part B5 of the Bankstown Development Control Plan 2015 specifies the following parking requirements that apply to Child Care Centres:

- 1 space per 4 children, and
- 2 additional spaces for the exclusive use of any associated dwelling

Application of this parking rate to the proposed Child Care Centre yields a total parking requirement of 10 spaces calculated as follows:

40 children @ 1 space per 4 children 10 spaces

The proposed development satisfies the DCP requirement with the provision of 10 spaces comprising 5 staff and 5 visitor spaces. Of the visitor parking provision, one space will be designed for disabled users.

<u>Subject to Council's approval</u>, it is recommended that the disabled space be utilised by both regular and disabled visitors to the Centre. In order to eliminate any confusion by visitors, the disabled linemarking and signposting will be removed however provision for wheelchair access will be retained.

In situations where use of the disabled space is required by a disabled user, the visitor will phone the Centre on the approach and request a staff member to go down to the basement and ensure that the space is vacant. To support this approach, reference is made to Section D3.5 of the Building Code of Australia (BCA) as part of the National Construction Code 2019 (NCC) which states the following:

D3.5 Accessible carparking

Accessible carparking spaces

(d) need not be identified within signage where there is a total of not more than 5 carparking spaces, so as to restrict the use of the car parking space only for people with a disability.



Carpark Compliance

The proposed carpark and access arrangements have been designed to satisfy the following requirements of the Australian Standard AS/NZS2890.1:2004 – "*Off-street Car Parking*":

- User class 1A "long-stay" staff parking spaces have a minimum length of 5.4m and width of 2.4m
- User class 3 "short-stay" visitor parking spaces have a minimum length of 5.4m and width of 2.6m
- An additional 0.3m has been provided for spaces adjacent to a wall or obstruction
- The access/manoeuvring aisle is a minimum 5.8m wide for User Class 1A and 3 parking facilities
- Pavement grades do not exceed 5% (1 in 20) in any direction
- The disabled shared zone provides the blind aisle extension for space 10
- The access driveway has a minimum width of 6.0m
- The grade of the access ramp for the first 6.0m into the site does not exceed 5% (1 in 20)
- A minimum headroom clearance of 2.2m has been provided
- Pedestrian sight line triangles have been provided in accordance with Figure 3.3

The disabled parking space has also been designed in accordance with the Australian Standard AS/NZS2890.6:2009 – "*Off-street parking for people with disabilities*" as follows:

- A 5.4m long x 2.4m wide dedicated (non-shared) parking space
- An adjacent shared area that is also 5.4m long x 2.4m wide
- A minimum headroom of 2.5m above the disabled spaces
- Pavement cross-falls in disabled spaces do not exceed 2.5% (1 in 40) in any direction

Deliveries to the Centre will be made with vans that will be capable of accessing the site during non-peak periods when the visitor spaces are under-utilised.

In the circumstances, it can be concluded that the proposed development has no unacceptable parking or access implications.



3. TRAFFIC ASSESSMENT

Road Hierarchy

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Services (RMS) is illustrated on Figure 3 and comprises the following:

State Roads	Regional Roads
Nil	Sphinx Avenue
	The River Road
	Gibson Avenue
	Uranus Road

As can be seen, Sphinx Avenue is a classified Regional Road performing a collector road function. It connects The River Road to the west with Gibson Avenue, Watson Road and ultimately Davies Road/Fairford Road to the east. It has a pavement width of approximately 12.5m with un-restricted parking along both alignments. Sphinx Avenue is restricted to a speed limit of 60km/h.

Cairo Avenue is an unclassified *Local Road* linking Sphinx Avenue to Cahors Road. It has a pavement width of approximately 7.3m with unrestricted parking permitted along both kerblines. Cairo Avenue is restricted to a speed limit of 50km/h.

Existing Traffic Conditions

An indication of existing traffic conditions on the road network serving the site is provided from a count of traffic activity at the Sphinx Avenue/Cairo Avenue intersection conducted between 7.00am - 9.00am and 4.00pm - 6.00pm on Monday 14th December 2020. The results of these counts of traffic activity are reproduced in Appendix B revealing that:

the AM peak period occurred between 8.00 – 9.00am. At that time, there were 564 vehicles per hour (vph) on Sphinx Avenue comprising 272vph eastbound and 292vph westbound vehicles. At that time, there were 48vph on Cairo Street comprising 5vph southbound and 45vph northbound vehicles









the PM peak period occurred between 4.15 – 5.15pm. At that time, there were 653vph on Sphinx Avenue comprising 300vph eastbound and 353vph westbound vehicles. At that time, there were 13vph on Cairo Street comprising 5vph heading southbound and 8vph northbound vehicles

Projected Traffic Generation

The Roads and Maritime Services publication "*Guide to Traffic Generating Developments*" (October 2002) specifies the following traffic generation rates for child care centres:

Morning Peak Period	0.8vtph per child
Evening Peak Period	0.7vtph per child

Application of these traffic generation rates to the proposed child care centre yields a traffic generation potential of 32vtph during the morning peak period and 28vtph during the evening peak period as follows:

Morning Peak Period	40 children @ 0.8vtph per child	32vtph (18 inbound, 14 outbound)
Evening Peak Period	40 children @ 0.7vtph per child	28vtph (12 inbound, 16 outbound)

	Staff			Visitors			
	Entry	Exit	Total	Entry	Exit	Total	
AM Peak (32vtph)	4	0	4	14	14	28	
PM Peak (28vtph)	0	4	4	12	12	24	

While there is no way of accurately predicting the origin and destination of staff and visitors, this assessment will assume traffic will approach and depart the site as follows:

- 50% to/from the east (Davies Road)
- 50% to/from the west (The River Road)

The assignment of the traffic generated by the proposed development is illustrated on Figure 4 for the morning and evening peak periods.



Traffic Implications

The main traffic implication of the proposed development in terms of road network capacity concerns the ability of traffic generated by the development to access the site and the impacts on through traffic along Sphinx Avenue. That effect can be assessed using the SIDRA traffic model and criteria for interpreting the results of SIDRA analysis are set out on the schedule reproduced in the following pages.

The results of the SIDRA analysis of the operating performance of the Sphinx Avenue Access Driveway are set out in Table 3.1 and on the SIDRA MOVEMENT SUMMARY SHEETS reproduced in Appendix C revealing that the intersection will operate satisfactorily with a good level of service and minimal delays.

	Level of Service	Degree of Saturation	Total Average Vehicle Delay (sec)
Proposed AM Peak	А	0.163	0.4
Proposed PM Peak	А	0.190	0.2

TABLE 3.1 – RESULTS OF SIDRA ANALYSIS OF THESPHINX AVENUE ACCESS DRIVEWAY

As the site has direct access to the higher order road network, there is no need for traffic generated by the proposal to travel on local residential streets. To that end, the proposed development will have no traffic-related environmental effect.

In the circumstances, it can be concluded that the proposed development has no unacceptable traffic implications.







Criteria for Interpreting Results of SIDRA Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
'E'	At capacity; at signals incidents will cause	At capacity and requires other control mode.
'F'	excessive delays. Roundabouts require other control mode. Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
А	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
Е	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.



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APPENDIX A

PLANS OF THE PROPOSED DEVELOPMENT







APPENDIX B

TRAFFIC SURVEY RESULTS



All Vehicles

R.O.A.R. DATA Reliable, Original & Authentic Results Ph.88196847, Mob.0418-239019

		TOTAL	73	85	112	104	109	108	165	192	948
EAST	Sphinx Ave	К	0	١	0	0	١	0	0	9E	38
EA	Sphin	Т	32	35	22	14	45	42	82	98	418
NORTH	Cairo A ve		0	1	1	0	1	0	0	0	3
ION	Cairc	R	0	0	0	١	١	2	0	1	5
WEST	Sphinx Ave	I	41	48	55	22	61	63	82	65	472
W	Sphin	L	0	0	1	5	0	1	1	4	12
		Time Per	0700 - 0715	0715 - 0730	0730 - 0745	0745 - 0800	0800 - 0815	0815 - 0830	0830 - 0845	0845 - 0900	Period End

۲.	Ave	<u>R</u> TOTAL	1 374	2 410	1 433	1 486	37 574
EAST	Sphinx A ve	ΓI	163	176	183	210	255
RTH	Cairo A ve	Ч	2	3	2	1	1
NORTH		ъ	1	2	4	4	4
SΤ	x A ve	ы	201	221	236	263	271
WES	Sphinx A ve	Г	9	9	7	7	9
8	2	Peak Per	0700 - 0800	0715 - 0815	0730 - 0830	0745 - 0845	0800 - 0900

574





	WEST	ST	NOF	NORTH	EAST	ST	
	Sphin	Sphinx Ave	Cairo	Cairo Ave	Sphinx Ave	x A ve	
Peak Per		Ч	R	Ē	μI	R	TOTAL
1600 - 1700	5	280	3	0	317	0	605
1615 - 1715	8	298	3	2	353	0	664
1630 - 1730	7	271	2	2	344	0	626
1645 - 1745	9	258	2	2	351	0	619
1700 - 1800	7	254	2	3	351	0	617









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APPENDIX C

SIDRA MOVEMENT SUMMARY SHEETS



MOVEMENT SUMMARY

V Site: [Sphinx Avenue and Site Access Driveway - AM Peak (Site Folder: General)]

Proposed AM Peak Site Category: (None) Give-Way (Two-Way)

Mov ID	Turn		INPUT VOLUMES		DEMAND FLOWS		Aver. Level of Delay Service		95% BACK OF QUEUE		Prop. Effective Que Stop	Effective		
		[Total veh/h	HV] %	[Total veh/h	HV] %	Satn v/c	sec	Oel vice	[Veh. veh	Dist] m	Que	Rate	Cycles	km/h
East:	Sphin	x Avenue	;						012301232					4.7.800.000
5	T1	292	3.0	292	3.0	0.163	0.1	LOSA	0.1	0.8	0.04	0.02	0.04	59.6
6	R2	11	0.0	11	0.0	0.163	6.9	LOS A	0.1	0.8	0.04	0.02	0.04	30.5
Appro	oach	303	2.9	303	2.9	0.163	0.3	NA	0.1	0.8	0.04	0.02	0.04	57.6
North	: Site	Access												
7	L2	9	0.0	9	0.0	0.022	0.8	LOS A	0.1	0.6	0.44	0.32	0.44	28.8
9	R2	9	0.0	9	0.0	0.022	4.4	LOSA	0.1	0.6	0.44	0.32	0.44	28.7
Approach		18	0.0	18	0.0	0.022	2.6	LOSA	0.1	0.6	0.44	0.32	0.44	28.7
West	: Sphir	nx Avenu	э											
10	L2	11	0.0	11	0.0	0.006	5.6	LOS A	0.0	0.0	0.00	0.58	0.00	53.6
11	T1	272	3.0	272	3.0	0.142	0.0	LOSA	0.0	0.0	0.00	0.00	0.00	59.9
Appro	oach	283	2.9	283	2.9	0.142	0.3	NA	0.0	0.0	0.00	0.02	0.00	59.7
All Vehic	les	604	2.8	604	2.8	0.163	0.4	NA	0.1	0.8	0.03	0.03	0.03	56.8

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

Delay Model: SIDRA Standard (Geometric Delay is included).

Queue Model: SIDRA Standard.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

V Site: [Sphinx Avenue and Site Access Driveway - PM Peak (Site Folder: General)]

Proposed PM Peak Site Category: (None) Give-Way (Two-Way)

Vehi	icle M	ovemen	t Perfo	rmance										
Mov ID	Turn	INP VOLU [Total veh/h		DEM FLO [Total veh/h		Deg. Satn v/c		Level of Service		ACK OF EUE Dist] m	Prop. E Que	Effective Stop Rate	Aver. No. Cycles	Aver. Speed km/h
East	: Sphir	ix Avenue	;											
5	T1	353	3.0	353	3.0	0.190	0.0	LOSA	0.1	0.5	0.02	0.01	0.02	59.8
6	R2	6	0.0	6	0.0	0.190	7.1	LOSA	0.1	0.5	0.02	0.01	0.02	30.6
Appr	oach	359	2.9	359	2.9	0.190	0.2	NA	0.1	0.5	0.02	0.01	0.02	58.9
North	n: Site	Access												
7	L2	8	0.0	8	0.0	0.022	1.0	LOS A	0.1	0.6	0.47	0.35	0.47	28.7
9	R2	8	0.0	8	0.0	0.022	5.3	LOSA	0.1	0.6	0.47	0.35	0.47	28.6
Approach		16	0.0	16	0.0	0.022	3.1	LOS A	0.1	0.6	0.47	0.35	0.47	28.6
West	t: Sphi	nx Avenu	e											
10	L2	6	0.0	6	0.0	0.003	5.6	LOSA	0.0	0.0	0.00	0.58	0.00	53.6
11	T1	300	3.0	300	3.0	0.157	0.0	LOS A	0.0	0.0	0.00	0.00	0.00	59.9
Appr	oach	306	2.9	306	2.9	0.157	0.1	NA	0.0	0.0	0.00	0.01	0.00	59.8
All Vehic	cles	681	2.9	681	2.9	0.190	0.2	NA	0.1	0.6	0.02	0.02	0.02	57.8

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

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HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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